

## Chapter 8

# Site effects in the cultural heritage areas of the City of Catania

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### Abstract

The city of Catania has been recognized as a typical Mediterranean city at high seismic risk. One of the peculiarities of this Mediterranean city is that there are many monuments and historical buildings. Recently some Churches were damaged by the moderate earthquake of December 13, 1990 with a magnitude  $M = 5.8$ . Because of that, to preserve the Churches from the damage which could be caused by a much stronger earthquake scenario ( $M = 7.0$ ), an analysis of local site effects in the areas where the Churches are located was performed. The analysis was made on the basis of geotechnical site characterization by specific investigations or by the available data from the database implemented in the Land Information System (LIS) of the city of Catania. The location of the 108 Churches was also implemented in the LIS, as well as the location of the 910 existing boreholes. So, by a simple query, the Geographical Information System (GIS) gives the nearest borehole to the Church area considered. On the basis of the given geotechnical properties of the foundation soil up to a depth of 30 m, the soil response analysis was performed by a non-linear 1-D model. The results show a great variability of response from site to site where the Churches are located. For each Church the time history of displacement, velocity and acceleration were evaluated as well as response spectra.

### 1 Introduction

Since the beginning of the Project: Detailed Scenarios and Actions for Seismic Prevention of Damage in the Urban Area of Catania (Maugeri [1]), Catania was recognized as a typical Mediterranean city at high seismic risk. One of the



peculiarities of this Mediterranean city is that there are many monuments and historical buildings. The Churches of Catania make up a unique chapter in the architectural scenario of our country; mainly built or rebuilt after the earthquake of 1693, in Catanese Late-Baroque style, they show architectural and structural characteristics that can be found in other parts of Italy only with great difficulty. The development plan is often rather articulated, a typical feature of the eighteenth century both in the search for a formal originality (Churches with a central layout) and because the buildings had to fit in with a town that was already heavily urbanized. Almost always the churches are close to other buildings and in many cases they are completely incorporated in the other buildings, being recognizable only by their facades that look out onto the public spaces; this is, for instance, the case of the Churches that are part of the monasteries and convents which are very numerous in the town (Cherubini *et al.* [2]). Recently the Sicilian Baroque of the Val di Noto has been included among the world cultural heritage by Unesco. The Baroque must be preserved from seismic damage due to any earthquake scenario (Priolo [3]) which could occur in Catania. In spite of the fact that the earthquake scenario could reach a magnitude  $M = 7.0$ , the moderate Sicilian earthquake of December 13, 1990 with a magnitude  $M = 5.8$  (Laurenzano and Priolo [4]) caused some damage to certain Churches. Because of that, to preserve the Churches from the damage which could be caused by a much stronger earthquake scenario, an analysis of local site effects in the areas where the churches are located, was performed. The analysis was made on the basis of geotechnical site characterization by specific investigations or by the available data from the database implemented in the Land Information System (LIS) of the city of Catania. The location of the 108 Churches was also implemented in the LIS, as well as the location of the 910 existing boreholes. So, by a simple query, the Geographical Information System (GIS), gives the nearest borehole to the Church area being considered. On the basis of the given geotechnical properties of the foundation soil up to a depth of 30 m, the soil response analysis was performed by a non-linear 1-D model.

## 2 Site characterization at the Church areas

The areas where the 108 Churches are located were inspected and localized in the map of Catania city. All the sites have been located and geo-settled in the GIS database (Fig. 1). In the GIS database are implemented the 910 existing boreholes (Grasso and Maugeri [5]), the geological map (Monaco *et al.* [6]), as well as the geotechnical map giving the shear waves velocity of the upper 30 m of soil (Grasso and Maugeri [7]).

For the Churches damaged by the December 13, 1990 earthquake, a detailed site investigation was made. In particular, at the Saint Nicola alla Rena Church site, boreholes, a Down-Hole test and geo-seismic survey were performed (Cavallaro *et al.* [8]). At the Purità Church site, boreholes, SPT and laboratory tests were performed (Cavallaro *et al.* [9]). Other specific site investigations, consisting of boreholes, SPT and laboratory tests were performed at S. Agata La

Vetere site. Fig. 2 shows a plan view of the Church with the borings location; Fig. 3 shows a photo of the main prospect of the Church.

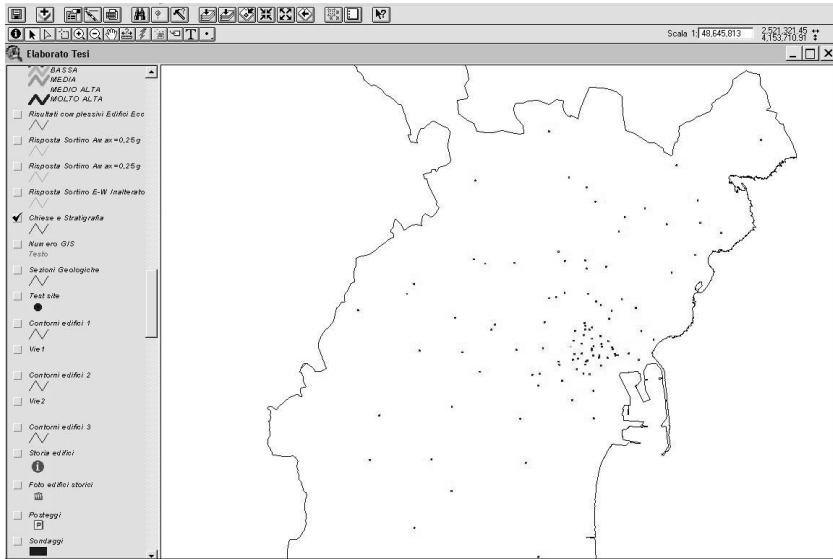


Figure 1: Localization of the 108 Churches in the GIS database of the city of Catania.

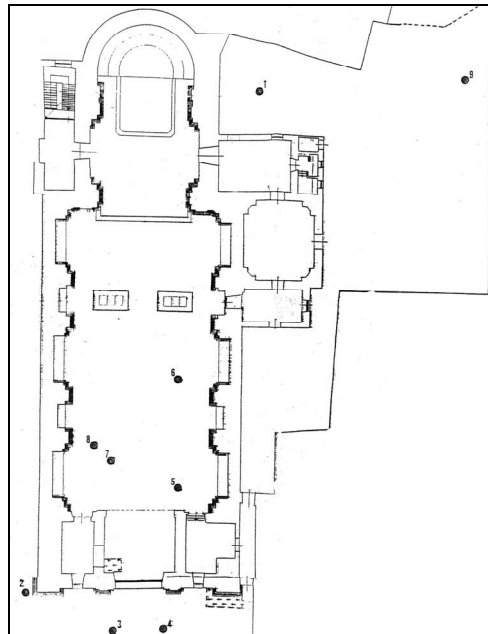


Figure 2: Plan view of the S. Agata La Vetere Church with borings locations.



Figure 3: Main prospect of the S. Agata La Vetere Church.

Similar specific investigations were made at Saint Sebastiano Church. The plan view of the church and the location of the borings are reported in Fig. 4. Among the other Churches considered, which could be damaged by the  $M = 7.0$  earthquake scenario, Figs. 5, 6 and 7 report respectively the Cathedral, the Collegiata and the Stesicoro Churches. For the site characterizations of these Churches and of the remaining ones by checking the GIS database, the nearest boring was considered. If the nearest boring was farther than 100 m from the Church site, a set of borings were considered and by the interpolation of data the stratigraphy at the site was established. This stratigraphy was checked by a query to the geological map and the geotechnical map, to obtain the final stratigraphy at the site.

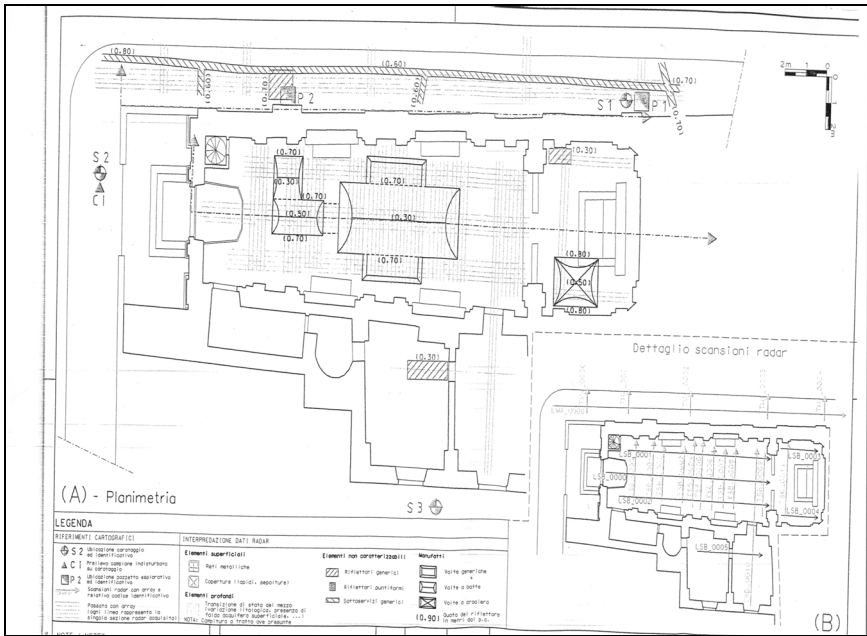


Figure 4: Plan view of the Saint Sebastiano Church with borings locations.



Figure 5: Main prospect of the Cathedral.



Figure 6: Main prospect of the Collegiata Church.



Figure 7: Main prospect of the Stesicoro Church.



The shear waves velocity was evaluated by a simple estimation related to the soil nature given by boreholes or by empirical correlations between the results of *in situ* tests (mainly SPT and CPT tests) and shear waves velocity. According to Cavallaro *et al.* [8] the following correlations for SPT tests were used:

a) Ohta and Goto [10]:

$$V_s = 54.33 \cdot (N_{\text{SPT}})^{0.173} \cdot \alpha \cdot \beta \cdot \left( \frac{Z}{0.303} \right)^{0.193} \quad (1)$$

where:  $V_s$  = shear waves velocity (m/s),  $N_{\text{SPT}}$  = number of blows SPT,  $Z$  = depth (m),  $\alpha$  = age factor (Olocene = 1.000, Pleistocene = 1.303),  $\beta$  = geological factor (clay = 1.000, sand = 1.086).

b) Yoshida and Motonori [11]:

$$V_s = \beta \cdot (N_{\text{SPT}})^{0.25} \cdot \sigma_{v0}^{0.14} \quad (2)$$

where:  $V_s$  = shear waves velocity (m/s),  $N_{\text{SPT}}$  = number of blows SPT,  $Z$  = depth (m),  $\sigma_{v0}$  = overburden pressure,  $\beta$  = geological factor (all soil = 55, fine sand = 49).

In a few cases, such as for the location of Saint Nicola alla Rena Church (Cavallaro *et al.* [8]), the estimation of shear waves velocity was checked by *in situ* dynamic tests: Down-Hole (DH) tests and by laboratory tests plus Resonant Column Tests (RCT) and Cyclic Loading Torsional Shear Tests (CLTST).

In addition the site and laboratory investigations, performed at some test sites and reported by Cavallaro and Maugeri [12] were considered.

For site response evaluation, the shear modulus at small strain ( $G_0$ ) was evaluated by the following expression according to:

c) Jamiolkowski *et al.* [13]:

$$G_0 = \frac{600 \cdot \sigma_m^{0.5} p_a^{0.5}}{e^{1.3}} \quad (3)$$

where:  $\sigma'_m = (\sigma'_v + 2 \cdot \sigma'_h)/3$ ;  $p_a = 1$  bar is a reference pressure;  $G_0$ ,  $\sigma'_m$  and  $p_a$  are reported in the same unit.

The results obtained by (3) were checked with the  $V_s$  measured by Down-Hole and Cross-Hole performed at test sites by means of the equation of elasticity:  $G_0 = \rho V_s^2$ , where  $\rho$  = mass density. The values of  $G_0$  were also checked by the Resonant Column Tests (RCT) and by Cyclic Loading Torsional Shear Tests (CLTST). The results are quite often in good agreement (Cavallaro *et al.* [8]); sometime laboratory tests give lower values of  $G_0$  than those obtained by *in situ* tests (Cavallaro *et al.* [9]).

RCT and CLTST performed at the test sites allow the measurement of the soil non-linearity which is very relevant for ground motion evaluation for the city of Catania (Grasso *et al.* [14]).

Soil non-linearity is taken into account by the following expressions according to Yokota *et al.* [15]:



$$\frac{G(\gamma)}{G_0} = \frac{1}{1 + \alpha\gamma(\%)^\beta}$$

$$D(\gamma)(\%) = \eta \cdot \exp \left[ -\lambda \cdot \frac{G(\gamma)}{G_0} \right] \quad (4)$$

in which:  $G_0$  = initial shear modulus;  $G(\gamma)$  = strain dependent shear modulus;  $\gamma$  = shear strain;  $\alpha$ ,  $\beta$  = soil constants;  $D(\gamma)$  = strain dependent damping ratio;  $\eta$ ,  $\lambda$  = soil constants.

For the clayey soil at the test site of via Stellata (Maugeri and Cavallaro [16]), the values are:  $\alpha = 11$ ,  $\beta = 1.119$ ,  $\eta = 31$ ,  $\lambda = 1.921$ .

For the stiff soil at the site of Saint Nicola alla Rena Church (Cavallaro *et al.* [8]), the values are:  $\alpha = 7.5$ ,  $\beta = 0.897$ ,  $\eta = 90$ ,  $\lambda = 4.5$ .

For the sandy soil at the test site of Piazza Palestro (Cavallaro and Maugeri [12]), the values are:  $\alpha = 6.9$ ,  $\beta = 1.0$ ,  $\eta = 23$ ,  $\lambda = 2.21$ .

The unit weight  $\gamma$  ranges between 18.3 and 20.5 KN/m<sup>3</sup>.

### 3 Site response analysis

The soil response analysis for the areas where the 108 Churches are located, has been performed by the 1-D non-linear code. As input earthquake at the bedrock we considered the scaled accelerograms of the December 13, 1990 Sicilian earthquake ( $M = 5.8$ ), recorded at the Sortino Station located on rock.

The maximum acceleration recorded at Sortino E-W component was 1.005 m/s<sup>-2</sup>. To simulate the scenario earthquake ( $M = 7.0$ ), the maximum acceleration was increased up to 0.25 g, which was about the maximum acceleration recorded at the Catania station during the December 1990 earthquake. Also the frequency was scaled, increasing the period by a factor of 1.333, simulating that, for a big earthquake, there will be a bigger period. The scaled earthquake input was applied at a depth of 30 m, as recommended by EC8 (CEN [17]), or at a shallow depth, where there is a contrast of shear waves velocity.

Considering that the city of Catania has not so many topographic irregularities, a 1-D code was used. By the way, there are not so many differences using 1-D or 2-D codes (Grasso *et al.* [14]). More relevant in soil response evaluations are the soil properties and the soil non-linearity. Soil non-linearity was taken into account considering the decrease of initial shear modulus and increase of damping with shear strength, as reported in the previous paragraph.

The 1-D code (Frenna and Maugeri [18]) gives the time history of displacement, velocity and acceleration. The time history at the surface, as well as the response spectra, were evaluated at each site where the 108 Churches are located. In Table 1 are reported the time history of accelerations and the response spectra at each site. The results show that there is different site amplification from site to site and so different response spectra, which shows the possible damage for each Church in relation to the site where it is located. In Fig. 8 are reported the locations of the 108 churches, with the different hazard for each Church.



Table 1: Time history of accelerations with the response spectra at each site.

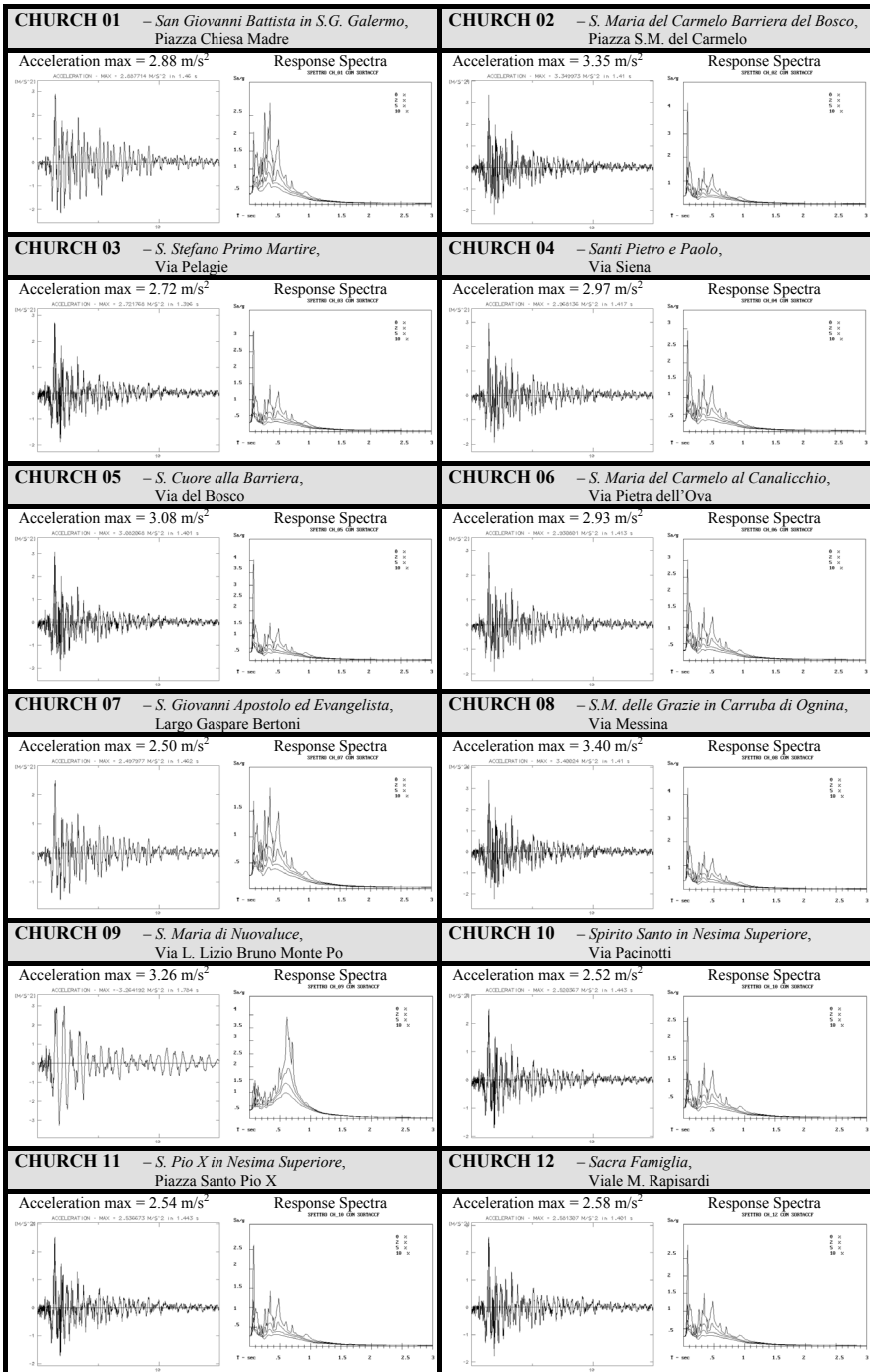


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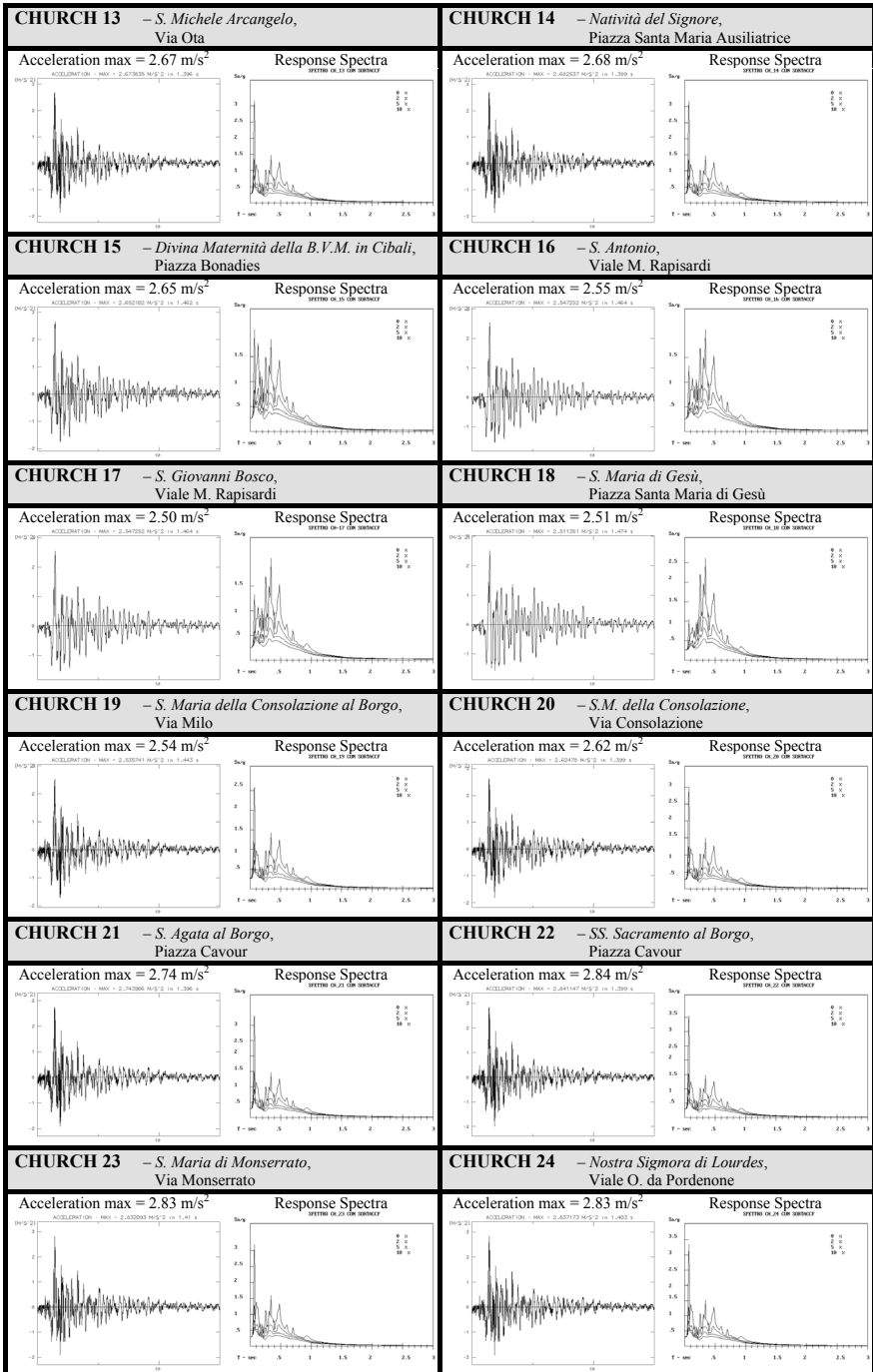


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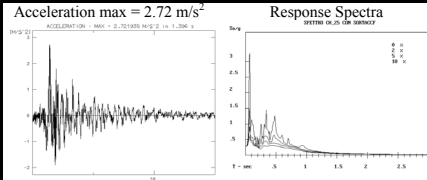
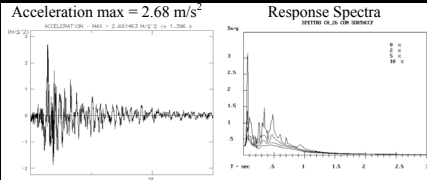
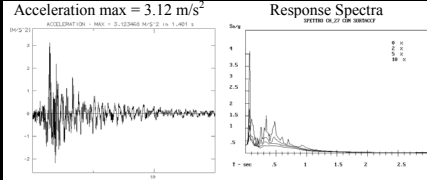
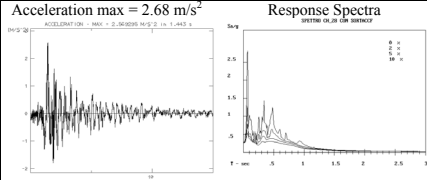
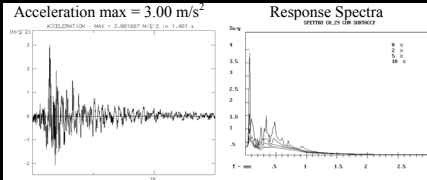
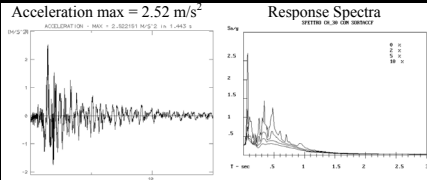
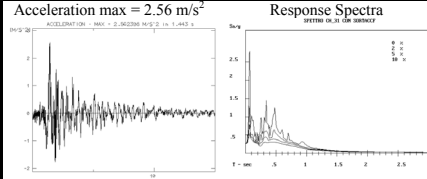
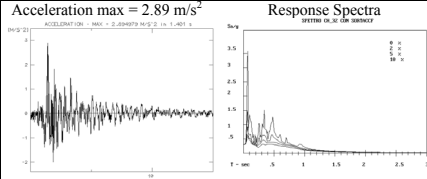
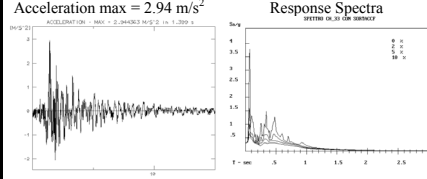
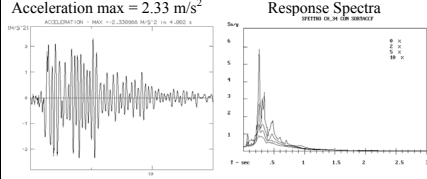
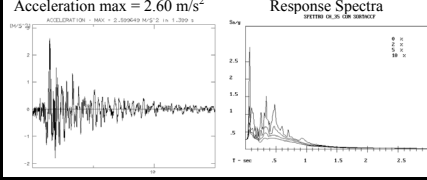
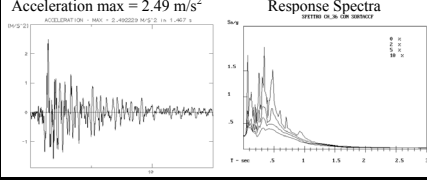
<p><b>CHURCH 25</b> – <i>S. Domenico Savio</i>, Piazza S. Domenico Savio</p> <p>Acceleration max = 2.72 m/s<sup>2</sup></p> 	<p><b>CHURCH 26</b> – <i>Cuore Immacolato della B.V.M. Picanello</i>, Viale V. Veneto</p> <p>Acceleration max = 2.68 m/s<sup>2</sup></p> 
<p><b>CHURCH 27</b> – <i>S.M. della Salute in Picanello</i>, Piazza Madonna della Salute</p> <p>Acceleration max = 3.12 m/s<sup>2</sup></p> 	<p><b>CHURCH 28</b> – <i>S. Lucia in Ognina</i>, Via Policastro</p> <p>Acceleration max = 2.68 m/s<sup>2</sup></p> 
<p><b>CHURCH 29</b> – <i>S. Giuseppe in Ognina</i>, Via Grasso Finocchiaro</p> <p>Acceleration max = 3.00 m/s<sup>2</sup></p> 	<p><b>CHURCH 30</b> – <i>S. Maria in Ognina</i>, Piazza Ognina</p> <p>Acceleration max = 2.52 m/s<sup>2</sup></p> 
<p><b>CHURCH 31</b> – <i>Maris Stella</i>, Via Anapo</p> <p>Acceleration max = 2.56 m/s<sup>2</sup></p> 	<p><b>CHURCH 32</b> – <i>S. Maria della Guardia in Ognina</i>, Via Zoccolanti</p> <p>Acceleration max = 2.89 m/s<sup>2</sup></p> 
<p><b>CHURCH 33</b> – <i>Cristo Re</i>, Corso Italia</p> <p>Acceleration max = 2.94 m/s<sup>2</sup></p> 	<p><b>CHURCH 34</b> – <i>S. Maria del Rosario in Nesima</i>, Via Monte Po</p> <p>Acceleration max = 2.33 m/s<sup>2</sup></p> 
<p><b>CHURCH 35</b> – <i>S. Teresa del Bambino Gesù</i>, Corso Indipendenza</p> <p>Acceleration max = 2.60 m/s<sup>2</sup></p> 	<p><b>CHURCH 36</b> – <i>S. Leone Vescovo</i>, Via S. Leone</p> <p>Acceleration max = 2.49 m/s<sup>2</sup></p> 



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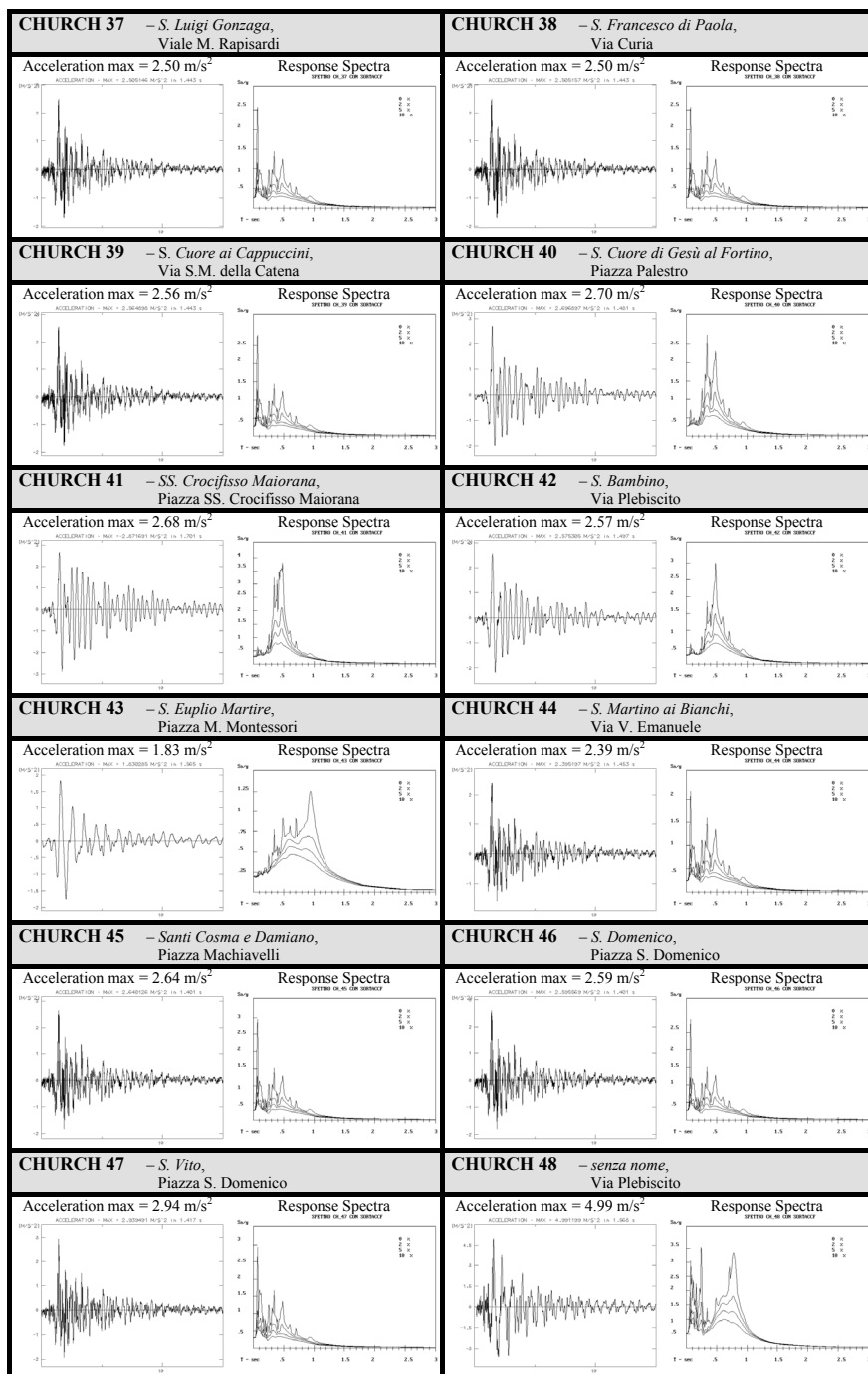


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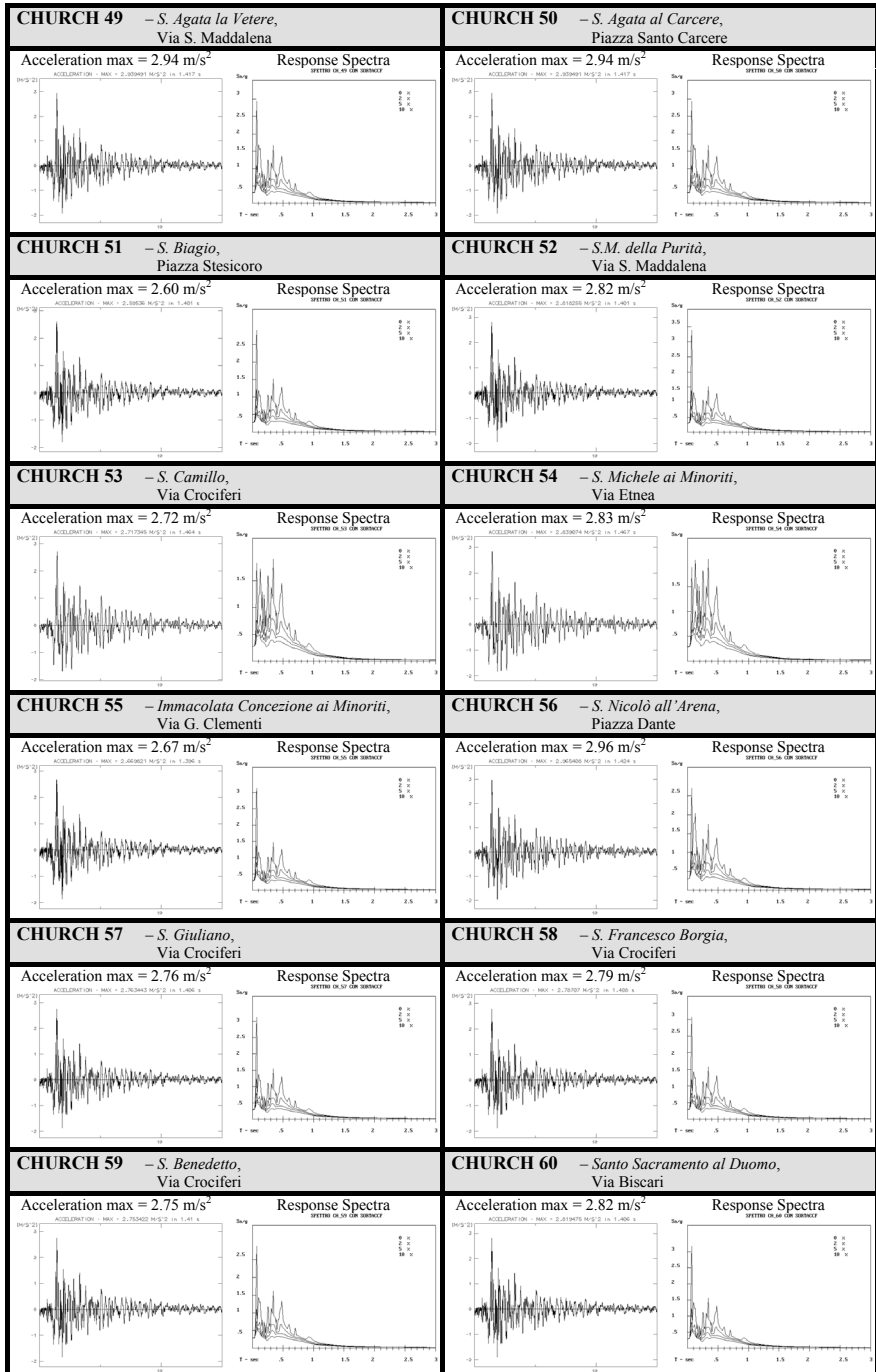


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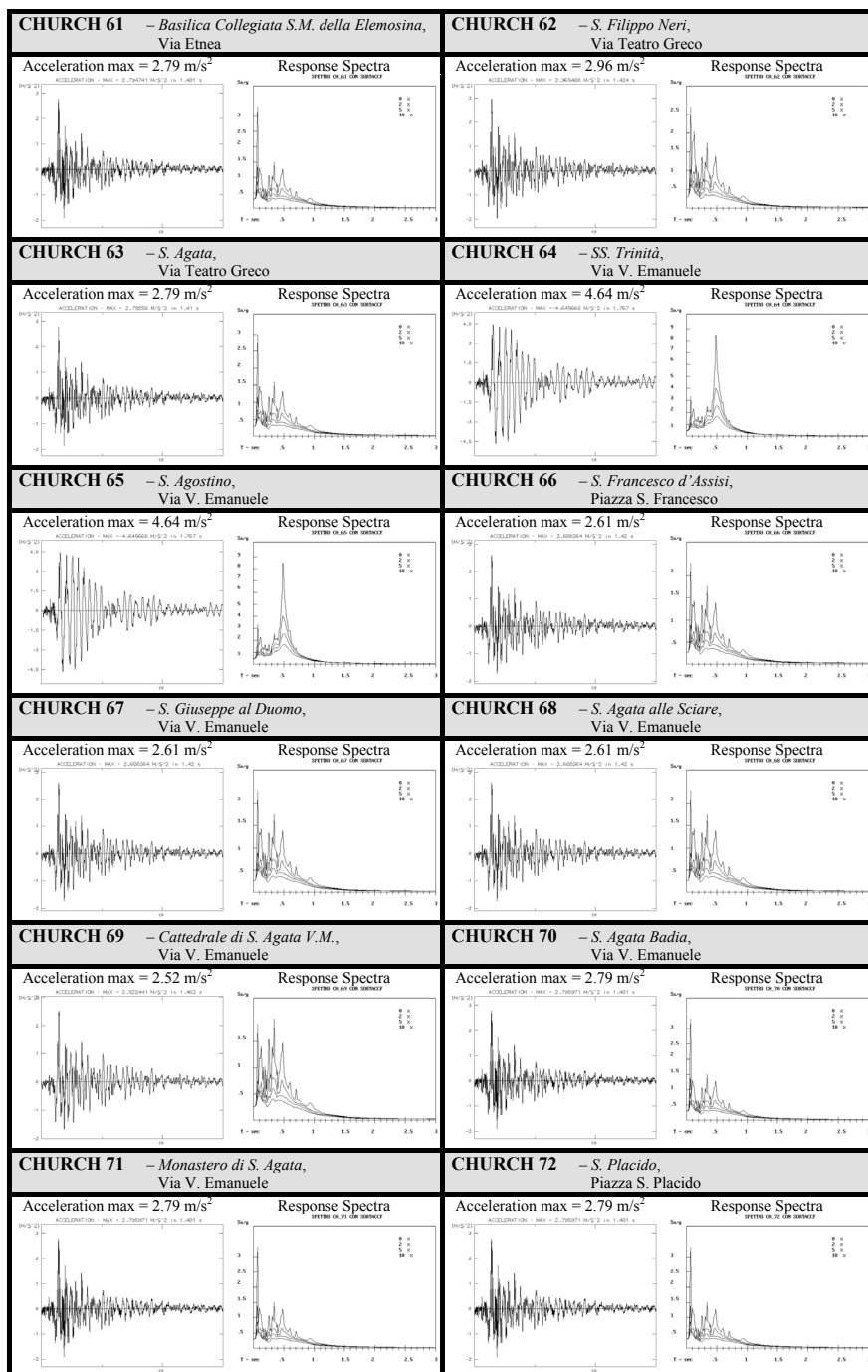


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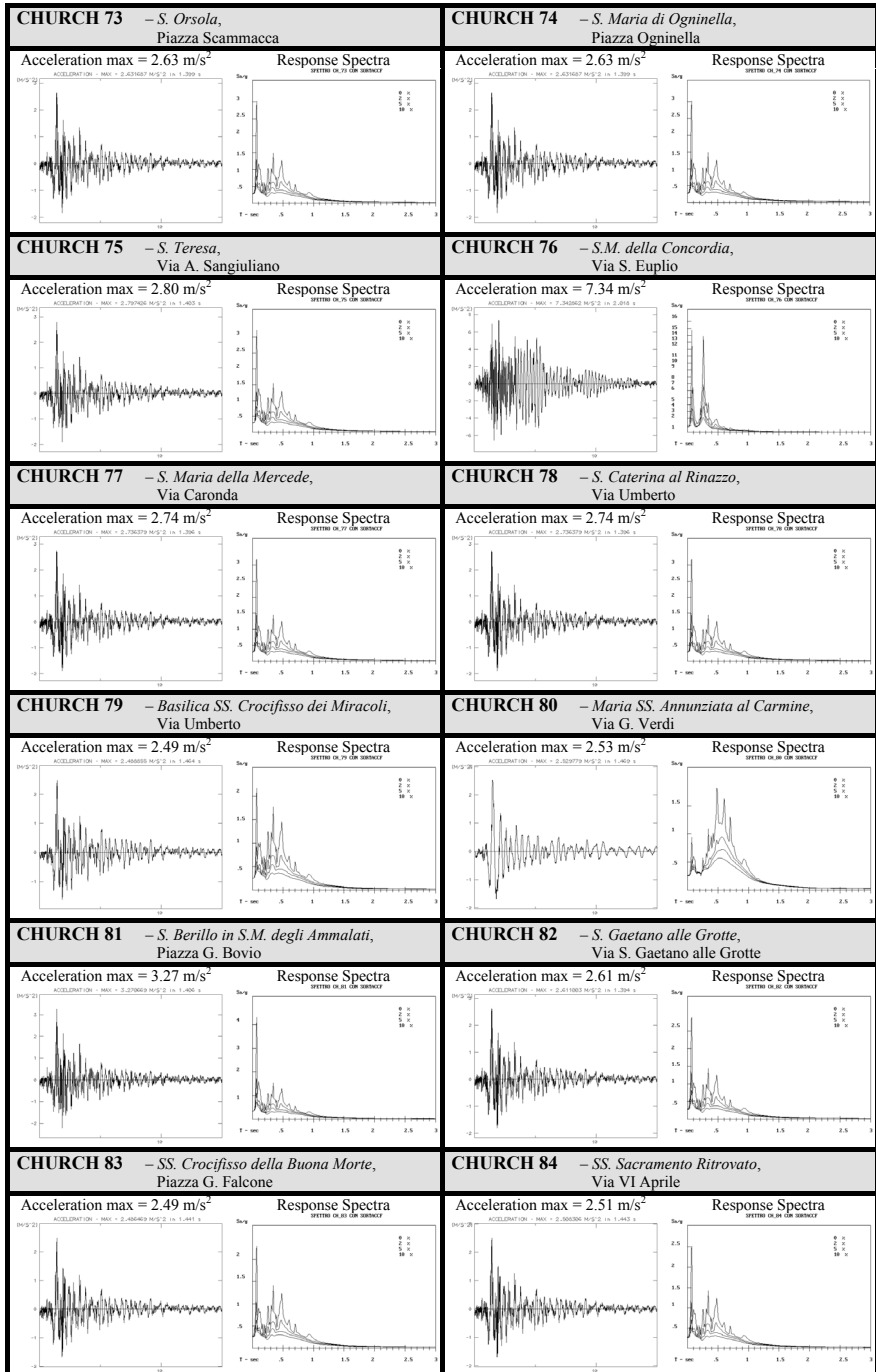


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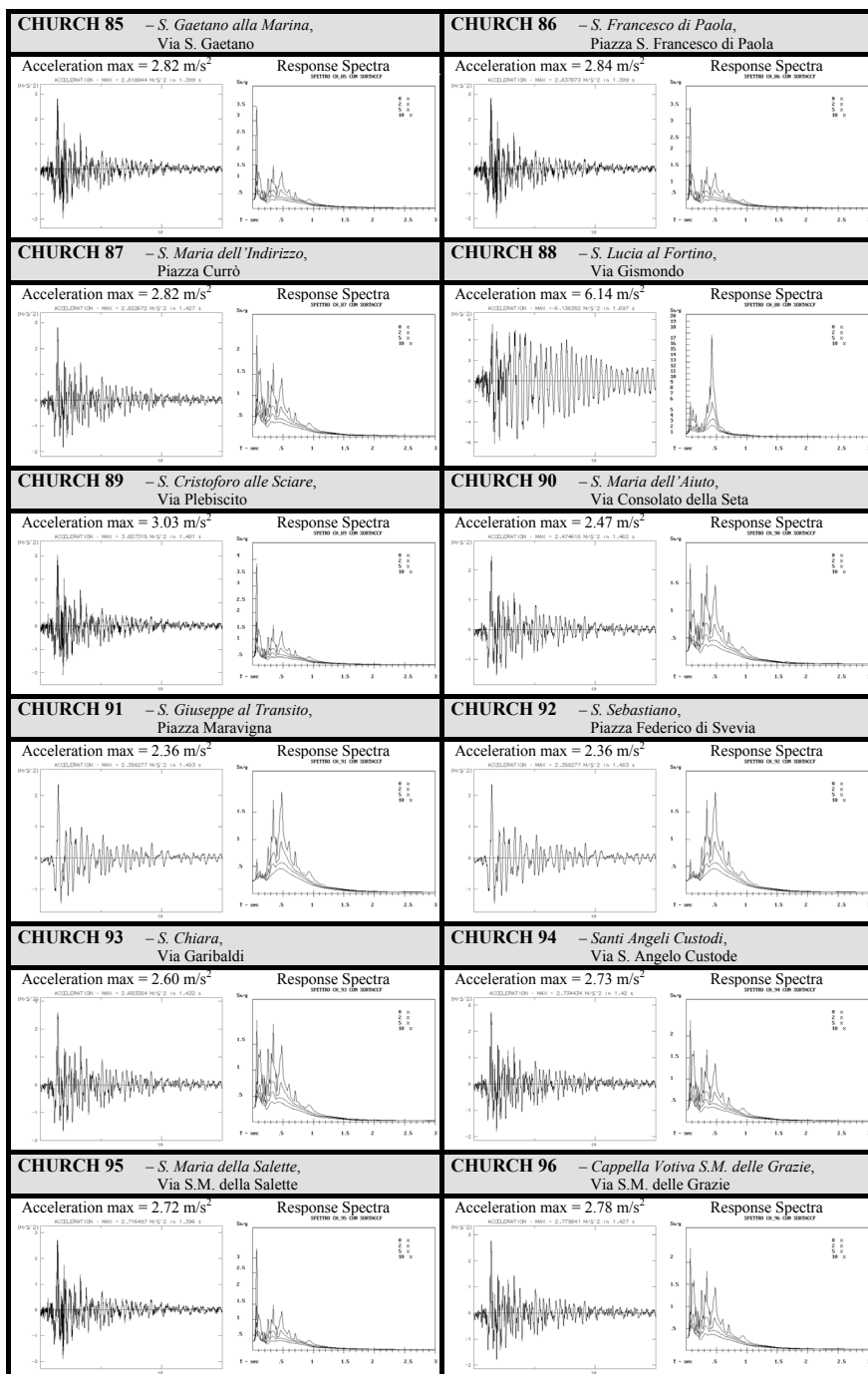
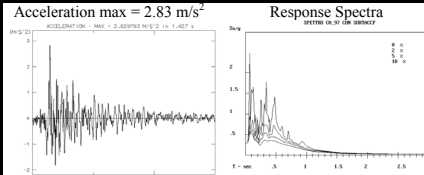
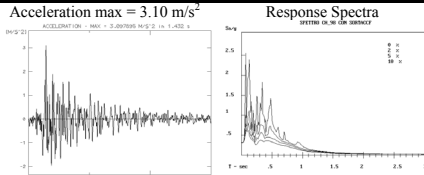
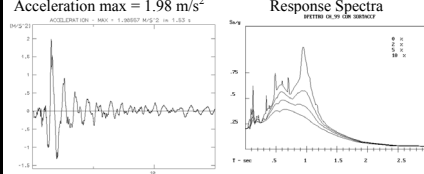
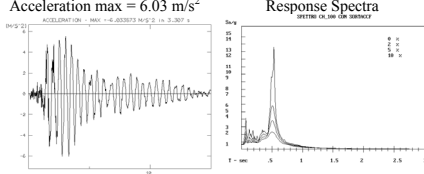
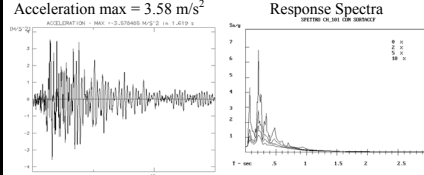
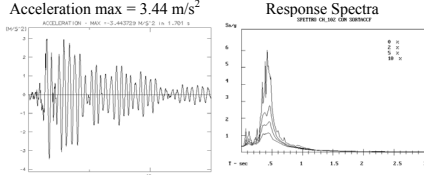
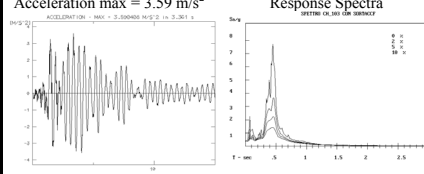
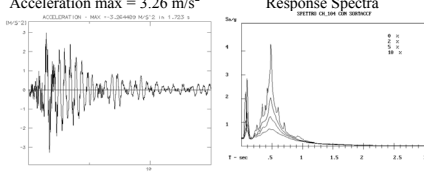
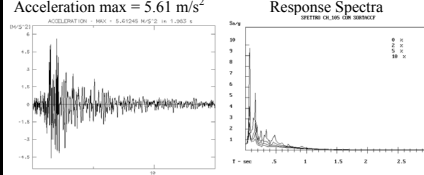
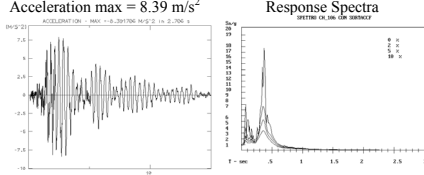
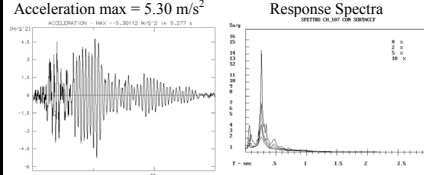
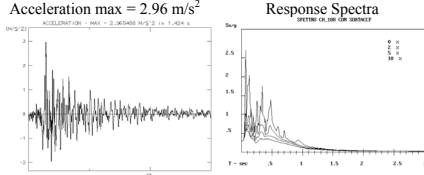




Table 1: Continued.

<p><b>CHURCH 97</b> – <i>S. Anna, Via S. Anna</i></p> <p>Acceleration max = 2.83 m/s<sup>2</sup></p> 	<p><b>CHURCH 98</b> – <i>B.V. Maria in Cielo Assunta alla Plaia, Piazza Caduti del Mare</i></p> <p>Acceleration max = 3.10 m/s<sup>2</sup></p> 
<p><b>CHURCH 99</b> – <i>Chiesa della Risurrezione, Cimitero Monumentale</i></p> <p>Acceleration max = 1.98 m/s<sup>2</sup></p> 	<p><b>CHURCH 100</b> – <i>S. Maria Ausiliatrice e S. Domenico Savio, Stradale S. Giorgio</i></p> <p>Acceleration max = 6.03 m/s<sup>2</sup></p> 
<p><b>CHURCH 101</b> – <i>B.M.V. del Carmelo e S.M. Goretti in S. Giorgio, Via dell'Idris</i></p> <p>Acceleration max = 3.58 m/s<sup>2</sup></p> 	<p><b>CHURCH 102</b> – <i>S. Chiara in Librino, Viale Monacale</i></p> <p>Acceleration max = 3.44 m/s<sup>2</sup></p> 
<p><b>CHURCH 103</b> – <i>Nostra Signora del SS. Sacramento in Librino, Via delle Susine</i></p> <p>Acceleration max = 3.59 m/s<sup>2</sup></p> 	<p><b>CHURCH 104</b> – <i>Risurrezione del Signore, Via Castagnola</i></p> <p>Acceleration max = 3.26 m/s<sup>2</sup></p> 
<p><b>CHURCH 105</b> – <i>Madonna del Divino Amore in Zia Lisa, Via Zia Lisa</i></p> <p>Acceleration max = 5.61 m/s<sup>2</sup></p> 	<p><b>CHURCH 106</b> – <i>S. Giuseppe al Pigno, Via delle Clementine</i></p> <p>Acceleration max = 8.39 m/s<sup>2</sup></p> 
<p><b>CHURCH 107</b> – <i>S. Giuseppe la Rena, Via S. Giuseppe la Rena</i></p> <p>Acceleration max = 5.30 m/s<sup>2</sup></p> 	<p><b>CHURCH 108</b> – <i>Madonna dell'Idra, Via Idra</i></p> <p>Acceleration max = 2.96 m/s<sup>2</sup></p> 



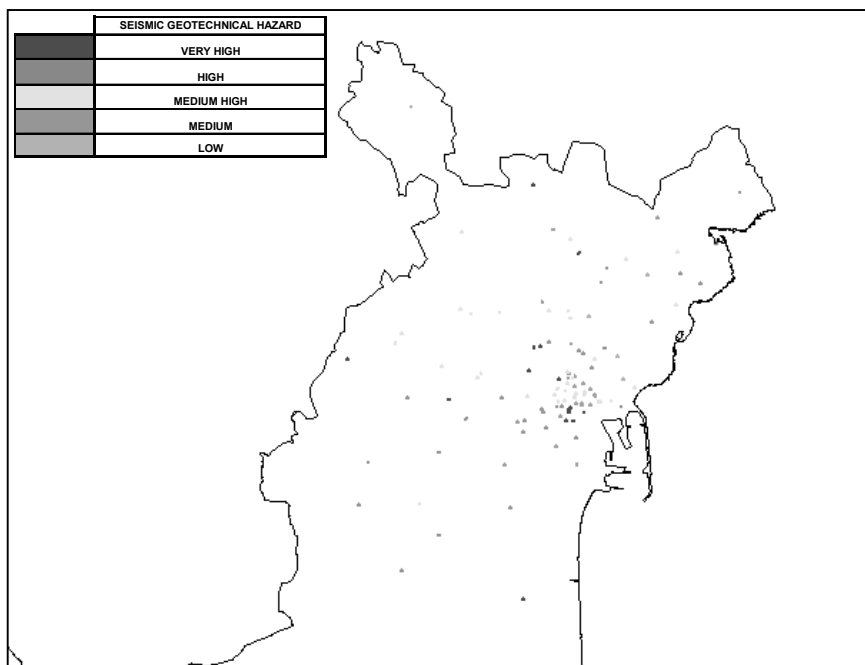


Figure 8: Locations of the 108 Churches, with the different seismic geotechnical hazard for each Church.

## 4 Conclusions

The seismic protection of the monuments located in Mediterranean cities is one of the main tasks to be achieved in the city of Catania. To this aim, the seismic hazard must be evaluated on the basis of an accurate site characterization of the areas where the churches are located. In some cases detailed site characterization was performed; in other cases the site characterization was reached by a query to the GIS database, including geology, borings and geotechnical soil properties. The site effects were evaluated by a 1-D code which takes into account soil non-linearity. The results show that seismic hazard changes very much from site to site. The soil response given for each church could be used to evaluate the design spectra, which can be used for the structural improvement of the Churches to resist against the earthquake scenario.

## References

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